

AS 4100 Supplement 1—1990

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**Steel structures—Commentary**  
**(Supplement to AS 4100—1990)**

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AS 4100 Supp1—1990

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### **Steel structures—Commentary (Supplement to AS 4100—1990)**

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## PREFACE

This Commentary is intended to be read in conjunction with AS 4100—1990, *Steel Structures*.

The purposes of this Commentary are to—

- (a) provide background reference material to the clauses in AS 4100—1990;
- (b) indicate the origins of particular requirements;
- (c) explain the application of certain clauses; and
- (d) provide some assistance in the use of AS 4100—1990.

AS 4100—1990 represents a comprehensive revision of AS 1250—1981, *SAA Steel Structures Code*, and AS 1511—1984, *SAA High-Strength Structural Bolting Code*. To put things into perspective, AS 1250—1981 essentially represented the technology of the 1970s. The considerable advances since then in materials and construction technology and the increased application of computers to modelling and analytical techniques has realized an improved understanding of both material and member behaviour in complete structures. This has led to more sophisticated design procedures which are now readily available to design-office staff.

While AS 4100—1990 inevitably reflects the abovementioned changes, a considerable amount of material and concepts has been retained from AS 1250—1981, particularly in those areas where the benefits of technical change seemed doubtful.

The clause numbers and titles used in this Commentary are the same as those in AS 4100—1990 except that the clause numbers are prefixed by the letter C, e.g. C7.2. In the Commentary, AS 4100—1990 is referred to as 'the Standard'.

Some Clauses in AS 4100—1990 are self-explanatory, and are therefore not discussed in this Commentary. In some cases, additional material is presented which is not directly related to a particular clause of AS 4100—1990.

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University of Sydney

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## CONTENTS

	<i>Page</i>
SECTION C1 SCOPE AND GENERAL	
C1.1 SCOPE .....	7
C1.2 REFERENCED DOCUMENTS .....	7
C1.3 DEFINITIONS .....	7
C1.4 NOTATION .....	7
C1.5 USE OF ALTERNATIVE MATERIALS OR METHODS .....	8
C1.6 DESIGN .....	8
C1.7 CONSTRUCTION .....	8
SECTION C2 MATERIALS	
C2.1 YIELD STRESS AND TENSILE STRENGTH USED IN DESIGN .....	9
C2.2 STRUCTURAL STEEL .....	9
C2.3 FASTENERS .....	9
C2.4 STEEL CASTINGS .....	9
REFERENCE TO SECTION C2 .....	9
SECTION C3 GENERAL DESIGN REQUIREMENTS	
C3.1 DESIGN .....	10
C3.2 LOADS AND OTHER ACTIONS .....	10
C3.3 STABILITY LIMIT STATE .....	10
C3.4 STRENGTH LIMIT STATE .....	10
C3.5 SERVICEABILITY LIMIT STATE .....	11
REFERENCES TO SECTION C3 .....	12
SECTION C4 METHODS OF STRUCTURAL ANALYSIS	
INTRODUCTION .....	14
C4.1 METHODS OF DETERMINING ACTION EFFECTS .....	14
C4.2 FORMS OF CONSTRUCTION ASSUMED FOR STRUCTURAL ANALYSIS .....	14
C4.3 ASSUMPTIONS FOR ANALYSIS .....	15
C4.4 ELASTIC ANALYSIS .....	15
C4.5 PLASTIC ANALYSIS .....	17
C4.6 MEMBER BUCKLING ANALYSIS .....	18
C4.7 FRAME BUCKLING ANALYSIS .....	19
REFERENCES TO SECTION C4 .....	19
SECTION C5 MEMBERS SUBJECT TO BENDING	
INTRODUCTION .....	21
C5.1 DESIGN FOR BENDING MOMENT .....	21
C5.2 SECTION MOMENT CAPACITY FOR BENDING ABOUT A PRINCIPAL AXIS .....	21
C5.3 MEMBER CAPACITY OF SEGMENTS WITH FULL LATERAL RESTRAINT .....	24
C5.4 RESTRAINTS .....	24
C5.5 CRITICAL FLANGE .....	26
C5.6 MEMBER CAPACITY OF SEGMENTS WITHOUT FULL LATERAL RESTRAINT .....	26
C5.7 BENDING IN A NON-PRINCIPAL PLANE .....	29
C5.8 SEPARATORS AND DIAPHRAGMS .....	29
C5.9 DESIGN OF WEBS .....	30
C5.10 ARRANGEMENT OF WEBS .....	30
C5.11 SHEAR CAPACITY OF WEBS .....	31
C5.12 INTERACTION OF SHEAR AND BENDING .....	34
C5.13 COMPRESSIVE BEARING ACTION ON THE EDGE OF A WEB .....	34
C5.14 DESIGN OF LOAD BEARING STIFFENERS .....	35
C5.15 DESIGN OF INTERMEDIATE TRANSVERSE WEB STIFFENERS .....	36
C5.16 DESIGN OF LONGITUDINAL WEB STIFFENERS .....	37
REFERENCE TO SECTION C5 .....	37

SECTION C6 MEMBERS SUBJECT TO AXIAL COMPRESSION	
INTRODUCTION	39
C6.1 DESIGN FOR AXIAL COMPRESSION	39
C6.2 NOMINAL SECTION CAPACITY	39
C6.3 NOMINAL MEMBER CAPACITY	40
C6.4 LACED AND BATTENED COMPRESSION MEMBERS	40
C6.5 COMPRESSION MEMBERS BACK TO BACK	42
C6.6 RESTRAINS	42
REFERENCES TO SECTION C6	43
SECTION C7 MEMBERS SUBJECT TO AXIAL TENSION	
C7.1 DESIGN FOR AXIAL TENSION	45
C7.2 NOMINAL SECTION CAPACITY	45
C7.3 DISTRIBUTION OF FORCES	45
C7.4 TENSION MEMBERS WITH TWO OR MORE MAIN COMPONENTS	46
C7.5 MEMBERS WITH PIN CONNECTIONS	46
REFERENCES TO SECTION C7	46
SECTION C8 MEMBERS SUBJECT TO COMBINED ACTIONS	
INTRODUCTION	48
C8.1 GENERAL	48
C8.2 DESIGN ACTIONS	48
C8.3 SECTION CAPACITY	48
C8.4 MEMBER CAPACITY	49
C8.5 TORSION	50
REFERENCES TO SECTION C8	56
SECTION C9 CONNECTIONS	
C9.1 GENERAL	57
C9.2 DEFINITIONS	59
C9.3 DESIGN OF BOLTS	59
C9.4 ASSESSMENT OF THE STRENGTH OF A BOLT GROUP	65
C9.5 DESIGN OF A PIN CONNECTION	66
C9.6 DESIGN DETAILS FOR BOLTS AND PINS	67
C9.7 DESIGN OF WELDS	67
C9.8 ASSESSMENT OF THE STRENGTH OF A WELD GROUP	73
C9.9 PACKING IN CONSTRUCTION	75
REFERENCES TO SECTION C9	75
SECTION C10 BRITTLE FRACTURE	
REFERENCES TO SECTION C10	78
SECTION C11 FATIGUE	
INTRODUCTION	79
C11.1 GENERAL	80
C11.2 FATIGUE LOADING	80
C11.3 DESIGN SPECTRUM	81
C11.4 EXEMPTION FROM FURTHER ASSESSMENT	82
C11.5 DETAIL CATEGORY	82
C11.6 FATIGUE STRENGTH	82
C11.7 EXEMPTION FROM FURTHER ASSESSMENT	82
C11.8 THICKNESS EFFECT	82
C11.9 FATIGUE ASSESSMENT	82
C11.10 PUNCHING LIMITATION	83
REFERENCES TO SECTION C11	83

	<i>Page</i>
SECTION C12 FIRE	
C12.1 REQUIREMENTS .....	84
C12.2 DEFINITIONS .....	84
C12.3 DETERMINATION OF PERIOD OF STRUCTURAL ADEQUACY .....	84
C12.4 VARIATION OF MECHANICAL PROPERTIES OF STEEL WITH TEMPERATURE .....	84
C12.5 DETERMINATION OF LIMITING STEEL TEMPERATURE .....	84
C12.6 DETERMINATION OF TIME AT WHICH LIMITING TEMPERATURE IS ATTAINED FOR PROTECTED MEMBERS .....	84
C12.7 DETERMINATION OF TIME AT WHICH LIMITING TEMPERATURE IS ATTAINED FOR UNPROTECTED MEMBERS .....	85
C12.8 DETERMINATION OF PSA FROM A SINGLE TEST .....	85
C12.9 THREE-SIDED FIRE EXPOSURE CONDITION .....	85
C12.10 SPECIAL CONSIDERATIONS .....	85
REFERENCES TO SECTION C12 .....	85
SECTION C13 EARTHQUAKE	
INTRODUCTION .....	86
C13.1 REQUIREMENTS .....	86
C13.2 DEFINITIONS .....	86
C13.3 SEISMIC ZONE .....	87
C13.4 EARTHQUAKE FORCES .....	87
C13.5 DESIGN REQUIREMENTS .....	87
C13.6 STRUCTURE IN SEISMIC ZONE A .....	88
C13.7 STRUCTURE IN SEISMIC ZONE 1 .....	88
C13.8 STRUCTURE IN SEISMIC ZONE 2 .....	88
C13.9 FABRICATION IN AN AREA OF PLASTIC DEFORMATION .....	89
REFERENCES TO SECTION C13 .....	89
SECTION C14 FABRICATION	
C14.1 GENERAL .....	90
C14.2 MATERIAL .....	90
C14.3 FABRICATION PROCEDURES .....	91
C14.4 TOLERANCES .....	93
C14.5 INSPECTION .....	95
C14.6 FABRICATION MATTERS AND CONTRACT DOCUMENTS .....	95
REFERENCES TO SECTION C14 .....	96
SECTION C15 ERECTION	
C15.1 GENERAL .....	97
C15.2 ERECTION PROCEDURES .....	97
C15.3 TOLERANCES .....	99
C15.4 INSPECTION OF BOLTED CONNECTIONS .....	101
C15.5 GROUTING AT SUPPORTS .....	101
C15.6 INSPECTION .....	101
C15.7 ERECTION MATTERS AND CONTRACT DOCUMENTS .....	101
REFERENCES TO SECTION C15 .....	101
SECTION C16 MODIFICATION OF EXISTING STRUCTURES	
C16.1 GENERAL .....	102
C16.2 MATERIALS .....	102
C16.3 CLEANING .....	102
C16.4 SPECIAL PROVISIONS .....	102
REFERENCES TO SECTION C16 .....	103

	<i>Page</i>
SECTION C17 TESTING OF STRUCTURES OR ELEMENTS	
C17.1 GENERAL .....	104
C17.2 DEFINITIONS .....	104
C17.3 TEST REQUIREMENTS .....	104
C17.4 PROOF TESTING .....	105
C17.5 PROTOTYPE TESTING .....	105
C17.6 REPORT OF TESTS .....	105
REFERENCE TO SECTION C17 .....	105
APPENDICES	
CA REFERENCED DOCUMENTS .....	107
CB SUGGESTED DEFLECTION LIMITS .....	107
CC CORROSION PROTECTION .....	107
CD ADVANCED STRUCTURAL ANALYSIS .....	108
CE SECOND-ORDER ELASTIC ANALYSIS .....	108
CF MOMENT AMPLIFICATION FOR A SWAY MEMBER .....	109
CG BRACED MEMBER BUCKLING IN FRAMES .....	109
CH ELASTIC RESISTANCE TO LATERAL BUCKLING .....	110
CI STRENGTH OF STIFFENED WEB PANELS UNDER COMBINED ACTIONS .....	112
CJ STANDARD TEST FOR EVALUATION OF SLIP FACTOR .....	112
CK INSPECTION OF BOLT TENSION USING A TORQUE WRENCH .....	112

## STANDARDS AUSTRALIA

**Australian Standard**  
**Steel structures—Commentary**

(Supplement to AS 4100—1990)

## SECTION C1 SCOPE AND GENERAL

by T. J. Hogan

**C1.1 SCOPE** The Standard sets out the minimum requirements for the limit states design, fabrication, erection, and modification of safe, serviceable and durable steel structures. There may be additional requirements not specifically covered that may also have to be considered by designers.

Road bridges are covered by the AUSTROADS Bridge Design Code (currently written in limit states format), while railway bridges are covered by the Bridge Design Manual of the ANZRC (currently not written in limit state format). AUSTROADS uses the Standard as the basis of its steel bridge design provisions. Both organizations are represented on the Committee (BD/1) that drafted the Standard.

Steel elements less than 3 mm in thickness are excluded for reasons of practicality and concern about corrosion, and because members from thinner material are usually cold-formed and then fall within the scope of AS 1538. In addition, the connections in elements less than 3 mm thick are better handled by the provisions of AS 1538 than by the Standard.

The limit of 450 MPa for the yield stress used in design stems from a lack of research data on steel grades above this value, and the applicability of all of the member design provisions for a higher design yield stress cannot be confirmed. Australian steel standards generally contain no steel grades with a specified yield stress above 450 MPa, with the exception of one grade (XF500) in AS 1594. Additional provisions to those in the standards may be required for steels of higher yield stress.

The Clause does not preclude the use of steels having a specified yield stress greater than 450 MPa provided that the yield stress used in design ( $f_y$ ) is limited to 450 MPa. Note, however, that the use of a steel having a specified yield stress greater than 450 MPa is specifically excluded from plastic design by Clause 4.5.2.

Hollow section members to AS 1163 are most commonly cold-formed, but have traditionally been designed using the previous editions of the Standard since they were for many years hot-formed. Tests carried out on members manufactured to AS 1163 confirm the applicability of the provisions of the Standard for such members. All other cold-formed members must be designed in accordance with AS 1538. Cold-formed hollow section members to AS 1163 with a wall thickness less than 3 mm should be designed in accordance with AS 1538, since the Clause excludes such members.

Composite steel-concrete members, including concrete encased steel members, should be designed using the provisions of AS 2327—SAA Composite Construction Code.

The Standard is not intended to be used for thin walled shell or plate structures since such structures are subject to failure modes not addressed in the Standard. It is, however, considered reasonable to design floor plates using the Standard. (See Introduction to Commentary on Section 5.)

**C1.2 REFERENCED DOCUMENTS** The Standards listed in Appendix A are subject to revision from time to time and the current issue should always be used. The currency of any standard may be checked with Standards Australia.

**C1.3 DEFINITIONS** Technical definitions are provided in the Clause. Some technical definitions which are applicable to only one Section are also given in the Section in which they are relevant. A number of the terms defined are common to other standards such as AS 1170.1 and AS 3600.

**C1.4 NOTATION** The change from the notation used in previous editions of the Standard to the ISO notation has been brought about by Standards Australia's policy of adopting the ISO recommendations on notation wherever practicable. This policy allows a unification of the notation used throughout all Australian Standards on structural design. The same basis has been used in AS 1170 and AS 3600.

The basis of the notation is generally in accordance with ISO 3898, Bases for Design of Structures — Notations — General Symbols.